

SECTION 8.0 – STREETS TABLE OF CONTENTS

SECTION 8.0 STREETS	2
8.1 INTRODUCTION	2
8.2 FUNCTION OF STREETS IN THE DRAINAGE SYSTEM	2
8.3 STREET CLASSIFICATIONS AND CAPACITY LIMITATIONS	2
8.3.1 STREET CLASSIFICATIONS.....	2
Table 8.3.1 – City of Greeley Street Classifications.....	2
8.3.2 STREET CAPACITY - INITIAL STORM.....	3
Table 8.3.2 – Street Capacity – Initial Storm -- Minor Storm.....	3
8.3.3 STREET CAPACITY - MAJOR STORM.....	4
Table 8.3.3 – Street Capacity for Major Storm Runoff.....	4
8.4 HYDRAULIC EVALUATION FOR STREET CAPACITY	4
8.4.1 ALLOWABLE STREET CAPACITY - INITIAL STORMS.....	4
8.4.2 ALLOWABLE STREET CAPACITY - MAJOR STORMS.....	5
8.4.3 RURAL STREETS (LOCAL, LOW-VOLUME STREETS WITHOUT CURB AND GUTTER).....	6
8.5 ALLOWABLE STREET CROSS-FLOW CONDITIONS	6
8.5.1 CROSS STREET FLOW AT INTERSECTIONS.....	6
Table 8.5.1 – Allowable Cross-Street Flow at Intersections.....	6
8.5.2 STREET OVERTOPPING.....	6
Table 8.5.2 – Allowable Culvert Overtopping.....	7
8.6 DESIGN EXAMPLE – Determination of street capacity	7
8.7 CHECKLIST	8

SECTION 8.0 STREETS

8.1 INTRODUCTION

The criteria presented in this section shall be used in the evaluation of the allowable drainage encroachment within public streets. The review of all submittals shall be based on the criteria herein.

8.2 FUNCTION OF STREETS IN THE DRAINAGE SYSTEM

Urban and rural streets, specifically the curb and gutter or the roadside ditches, are part of the storm drainage system. When the drainage in the street exceeds allowable limits, a storm drain system or an open channel is required to convey the excess flows. However, the primary function of the urban street system is for traffic movement, and, therefore, the drainage function is secondary and must not interfere with the traffic function of the street.

Design criteria for collecting and moving runoff water on public streets are based on a reasonable frequency and magnitude of traffic interference. Depending on the character of the street, certain traffic lanes can be inundated during specific design storm runoff events. The primary function of the streets during the initial storm runoff event is to convey the nuisance flows quickly and efficiently to the storm drain or open channel drainage without interference with traffic movement. During the major storm runoff event the function of the streets is to provide a passageway for the flood flows with minimal damage to the urban environment, and passage of emergency vehicles.

8.3 STREET CLASSIFICATIONS AND CAPACITY LIMITATIONS

8.3.1 STREET CLASSIFICATIONS

The streets in the City are classified for drainage use according to the average daily traffic (ADT) for which the street is designed and the adopted City classifications. The larger the ADT, the more restrictive the allowable drainage encroachment into the driving lanes is. Refer to the City of Greeley Standard Details and "Street Design Criteria" for specific dimensions and cross sections of each street classification. The adopted City classifications are summarized in the following table:

TABLE 8.3.1 – CITY OF GREELEY STREET CLASSIFICATIONS

CITY OF GREELEY STREET CLASSIFICATIONS		
Classification	Width (Flow line to Flow line)	City of Greeley Standard Detail No.*
Local - Low Volume*	No Curb & Gutter Shoulder Only w/Ditch	S-1
Local – Commercial / Industrial & Residential	40'	S-2
Local – Performance Option 14	34'	S-2-14

Minor Collector	52'	S-3
Major Collector	64'	S-4
Minor Arterial	64'	S-5
Major Arterial (with Raised Median)	2 Lanes at 33' each	S-6

* Reference: See City of Greeley Street Design Criteria Manual for Standard Detail Nos. S-1 to S-6 plus street options.

8.3.2 STREET CAPACITY - INITIAL STORM

The street capacity for initial storm runoff events is determined by the limitations set forth below:

TABLE 8.3.2 – STREET CAPACITY – INITIAL STORM -- MINOR STORM

STREET CAPACITY FOR INITIAL STORM or MINOR STORM RUNOFF		
Street Classification	Curb Overtopping Allowed	Maximum Pavement Encroachment
Local ¹ Low Volume	No	Flow may spread to crown of street
Local Commercial / Industrial & Residential & Performance Option 14 w/ vertical curb	No	Flow may spread to crown of street
Local Commercial / Industrial & Residential & Performance Option 14 w/ rollover curb	Yes 5" max above gutter FL	Flow may spread to crown of street
Minor Collector	No	Flow must leave a minimum 10' wide center lane open
Major Collector	No	Flow must leave a minimum 10' wide center lane open
Minor Arterial	No	Flow must leave a minimum of one traffic lane open each direction

Major Arterial	No	Flow must leave a minimum of one traffic lane open each direction
<p>*Note: ¹For Local - Low Volume Streets see Section 5.4 of these Criteria for the design and capacity of roadside ditches.</p>		

8.3.3 STREET CAPACITY - MAJOR STORM

The street capacity during major storm events is determined by the limitations set forth below:

TABLE 8.3.3 – STREET CAPACITY FOR MAJOR STORM RUNOFF

STREET CAPACITY FOR MAJOR STORM RUNOFF ¹			
Street Classification	Maximum Depth At Gutter Flow line	Maximum Depth at Crown	Allowable Inundation
Local ² , Local Commercial / Industrial / Residential / Option 14	18"	N/A	No inundation at groundline ³
Collector	18"	N/A	No inundation at groundline ³
Arterial	18"	6"	No inundation at groundline ³
<p>Notes: ¹Most restrictive condition shall control design ²For Local - Low Volume Streets see Section 5.4 of these Criteria for the design and capacity of roadside ditches. ³Includes inundation of residential dwellings, public, commercial and industrial buildings</p>			

8.4 HYDRAULIC EVALUATION FOR STREET CAPACITY

8.4.1 ALLOWABLE STREET CAPACITY - INITIAL STORMS

The determination of the Allowable Street Capacity shall be based on the following procedure: determine the Theoretical Capacity based on the street cross section; compute the street flow; then, apply the appropriate reduction factor to calculate the Allowable Street Capacity.

Based on the Maximum Pavement Encroachment for the various street classifications presented in Section 8.3, the Theoretical Capacity of each street section is calculated using the Modified Manning's formula shown below:

$$Q = (0.56) (Z/n) S^{1/2} d^{8/3}$$

Equation 8.4.1

Where: Q = discharge in cfs

Z = 1/S_x, where S_x is the cross slope of the pavement (ft/ft)

d = depth of water at face of curb (feet)

S = longitudinal grade of street (ft/ft)

n = Manning's roughness coefficient

Note: This equation does not pertain to streets with borrow ditches. Also, the solution to the above equation can be obtained through the use of the nomograph (Figure 8-1 and included for information only).

The Allowable Capacity of a Street Section is then calculated by multiplying the Theoretical Capacity by the appropriate reduction factor found in Figure 8-2. The purpose of the reduction factor is to account for various street conditions, which decrease the street capacity. These conditions may include street overlays, parked vehicles, debris and hail accumulation, and deteriorated pavement.

The Designer will find the Allowable Street Capacity already calculated in Table 8-1 for several of the standard, symmetrical street sections. The calculations were performed for various allowable flow depths and street slopes. A Manning's n-value of 0.016 was used for the calculations at all street slopes.

Other accepted street options used within the City are not included in the Figures and Tables. The Designer shall calculate the Theoretical Capacity using equation 8.4.1, then calculate the Allowable Street Capacity by multiplying the Theoretical Capacity obtained, by the appropriate reduction factor found in Figure 8-2. These street options shall meet the requirements specified in the Street Capacity for Initial Storm Runoff depending on ADT and Street Classification (Contact Traffic Division for this information). These calculations shall be included in the Drainage Report.

The Allowable Street Capacity will also need to be reduced if non-symmetrical street sections are encountered. Street capacity calculations at critical locations of non-symmetrical street sections shall be submitted in the Drainage Report.

8.4.2 ALLOWABLE STREET CAPACITY - MAJOR STORMS

The street capacity for the major storm is determined by the depth and inundation limits set forth in Section 8.3.3. The Allowable Street Capacity is found by using the same procedure outlined in Section 8.4.1 with one exception due to the addition of grass areas. A weighted Manning's n for the entire roadway cross-section will be used in Equation 8.4.1 to find the maximum theoretical flowrate – Q.

Again, the Designer will find the Allowable Street Capacity already calculated in Table 8-1 for several of the City's standard, symmetrical street sections. A Manning's value of 0.016 for the pavement and sidewalk areas and 0.033 for the grass area was used to determine capacity. The maximum allowable depth at the gutter flowline is 18 inches. The street capacity criteria for both the initial and major storms are graphically displayed by Figures 8-3 and 8-4.

For non-symmetrical streets, such as shown in Figure 8-13, street capacity calculations shall be performed at all critical locations and shall be submitted to the City for review. The computed street capacity must never exceed the allowable street capacity presented in these criteria.

8.4.3 RURAL STREETS (LOCAL, LOW-VOLUME STREETS WITHOUT CURB AND GUTTER)

Rural streets are characterized by the use of roadside ditches instead of curb and gutters. The capacity is limited by the depth in the ditch and the maximum flow velocity. Refer to Section 5.4 for the design and capacity of roadside ditches.

8.5 ALLOWABLE STREET CROSS-FLOW CONDITIONS

8.5.1 CROSS STREET FLOW AT INTERSECTIONS

Cross street flow normally occurs at converging street intersections where the flow must cross from one side to the other in either a cross pan (where allowed) or across the street crown. The restrictions for flow depth at intersections are set forth below:

TABLE 8.5.1 – ALLOWABLE CROSS-STREET FLOW AT INTERSECTIONS

ALLOWABLE CROSS STREET FLOW AT INTERSECTIONS		
Street Classification	Initial Storm or Minor Storm Runoff	Major Storm Runoff
Local w/ vertical curb	Maximum 6" Depth at Street Crown or in Cross Pan	Maximum 18" Depth Above Gutter Flow line
Local w/ rollover curb	Maximum 5" Depth at Street Crown or in Cross Pan	Maximum 18" Depth Above Gutter Flow line
Collector	Maximum 6" Depth Above Cross Pan Flow line (Where Cross Pan is Allowed)	Maximum 18" Depth Above Gutter Flow line
Minor Arterial	None Allowed	Maximum 6" Depth Above Crown
Major Arterial	None Allowed	Maximum 6" Depth Above Crown

8.5.2 STREET OVERTOPPING

In locations of culvert crossings, the opportunity for the flow in the drainage way to exceed the road culvert capacity and subsequently overtop the crown of the street must be investigated. The restrictions for street overtopping are set forth below:

TABLE 8.5.2 – ALLOWABLE CULVERT OVERTOPPING

ALLOWABLE CULVERT OVERTOPPING		
Street Classification	10-Year Storm Maximum Depth	Major Storm Maximum Depth ¹
Local	None	18" At The Gutter Flow line
Local w/Roadside Ditch	None	6" At The Street Crown
Collector & Minor Arterial	None	6" At The Street Crown
Major Arterial	None	No Overtopping Allowed For bridges the minimum clearance between the low chord and the EGL shall be 6".
¹ . The maximum headwater for the 100-year design flows shall be 1.5 times the culvert diameter or 1.5 times the rise dimension for pipe shapes other than round.		

8.6 DESIGN EXAMPLE – DETERMINATION OF STREET CAPACITY

GIVEN:

Street with a traffic classification of "Minor Collector" and a slope of 1.0 percent.

FIND:

Maximum allowable capacity for initial and major storm.

SOLUTION:

STEP 1: Determine the allowable depth:

From Section 8.3, for a Minor Collector, the maximum depth at the curb (without overtopping) would be 6" for the initial storm.

STEP 2: Determine the allowable initial storm gutter capacity:

From Table 8-1, for a "Minor Collector" with an allowable depth of 0.50 feet and a slope of 1.0 percent, read the allowable gutter capacity of 8.6 cfs.

STEP 3: Determine the allowable major storm street capacity:

From Table 8-1, for a "Minor Collector" with a slope of 1.0 percent, read the allowable capacity of 610 cfs for the full street section, assuming the street is symmetrical.

8.7 CHECKLIST

To aid the Designer and Reviewer, the following checklist has been prepared:

1. Determine the street classification first and then the allowable flow depth and gutter capacity.
2. Use the flattest street slope to determine the gutter capacity.
3. To calculate the allowable street flow, use the appropriate reduction factor (F) to calculate the allowable gutter capacity.
4. Check for non-symmetrical street evaluation.
5. Check for cross-flow conditions at intersections and allowable culvert overtopping depths.
6. Storm drains required when gutter capacity is exceeded.
7. Check adequacy of downstream facilities.

Local-Residential w/ 3-3/8" tall rollover curb						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity w/ 5" of water above flow line (cfs)	Allowable Capacity w/ 5" of water above flow line (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	5.8	2.9	459	230
0.005	0.650	0.650	6.5	4.2	514	334
0.006	0.800	0.800	7.1	5.7	563	450
0.008	0.800	0.800	8.2	6.6	650	520
0.009	0.800	0.800	8.7	7.0	689	551
0.010	0.800	0.800	9.2	7.4	726	581
0.020	0.800	0.700	13.0	10.4	1027	719
0.040	0.610	0.500	18.4	11.2	1453	727
0.060	0.410	0.375	22.6	9.3	1779	667
0.080	0.280	0.270	26.1	7.3	2054	555

Local-Commercial / Industrial & Residential w/ vertical curb						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity (cfs)	Allowable Capacity (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	6.9	3.5	386	193
0.005	0.650	0.650	7.7	5.0	432	281
0.006	0.800	0.800	8.4	6.7	473	378
0.008	0.800	0.800	9.7	7.8	547	438
0.009	0.800	0.800	10.3	8.2	580	464
0.010	0.800	0.800	10.8	8.6	611	489
0.020	0.800	0.700	15.3	12.2	864	605
0.040	0.610	0.500	21.7	13.2	1222	611
0.060	0.410	0.375	26.6	10.9	1497	561
0.080	0.280	0.270	30.7	8.6	1728	467

Minor Collector						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity (cfs)	Allowable Capacity (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	6.8	3.4	482	241
0.005	0.650	0.650	7.6	4.9	539	350
0.006	0.800	0.800	8.4	6.7	591	473
0.008	0.800	0.800	9.7	7.8	682	546
0.009	0.800	0.800	10.2	8.2	724	579
0.010	0.800	0.800	10.8	8.6	763	610
0.020	0.800	0.700	15.3	12.2	1079	755
0.040	0.610	0.500	21.6	13.2	1525	763
0.060	0.410	0.375	26.4	10.8	1868	701
0.080	0.280	0.270	30.5	8.5	2157	582

TABLE 8-1

Major Collector						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity (cfs)	Allowable Capacity (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	6.8	3.4	527	264
0.005	0.650	0.650	7.6	4.9	589	383
0.006	0.800	0.800	8.4	6.7	646	517
0.008	0.800	0.800	9.7	7.8	745	596
0.009	0.800	0.800	10.2	8.2	791	633
0.010	0.800	0.800	10.8	8.6	833	666
0.020	0.800	0.700	15.3	12.2	1179	825
0.040	0.610	0.500	21.6	13.2	1667	834
0.060	0.410	0.375	26.4	10.8	2041	765
0.080	0.280	0.270	30.5	8.5	2357	636

Minor Arterial						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity (cfs)	Allowable Capacity (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	6.8	3.4	339	170
0.005	0.650	0.650	7.6	4.9	379	246
0.006	0.800	0.800	8.4	6.7	416	333
0.008	0.800	0.800	9.7	7.8	480	384
0.009	0.800	0.800	10.2	8.2	509	407
0.010	0.800	0.800	10.8	8.6	537	430
0.020	0.800	0.700	15.3	12.2	759	531
0.040	0.450	0.450	21.6	9.7	1073	483
0.060	0.275	0.275	26.4	7.3	1315	362
0.080	0.175	0.175	30.5	5.3	1518	266

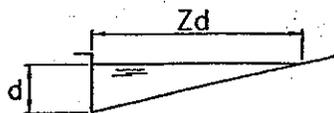
Major Arterial						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity (cfs)	Allowable Capacity (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	6.8	3.4	367	184
0.005	0.650	0.650	7.6	4.9	410	267
0.006	0.800	0.800	8.4	6.7	450	360
0.008	0.800	0.800	9.7	7.8	519	415
0.009	0.800	0.800	10.2	8.2	551	441
0.010	0.800	0.800	10.8	8.6	581	465
0.020	0.800	0.700	15.3	12.2	821	575
0.040	0.450	0.450	21.6	9.7	1161	522
0.060	0.275	0.275	26.4	7.3	1422	391
0.080	0.175	0.175	30.5	5.3	1642	287

TABLE 8-1A

Performance Option 14 w/ 3-3/8" tall rollover curb						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity w/ 5" of water above flow line (cfs)	Allowable Capacity w/ 5" of water above flow line (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	5.4	2.7	434	217
0.005	0.650	0.650	6.1	4.0	485	315
0.006	0.800	0.800	6.6	5.3	531	425
0.008	0.800	0.800	7.7	6.2	613	490
0.009	0.800	0.800	8.1	6.5	650	520
0.010	0.800	0.800	8.6	6.9	686	549
0.020	0.800	0.700	12.1	9.7	970	679
0.040	0.610	0.500	17.1	10.4	1371	686
0.060	0.410	0.375	20.9	8.6	1679	630
0.080	0.280	0.270	24.2	6.8	1939	524

Performance Option 14 w/ vertical curb						
Gutter Slope (ft/ft)	Reduction Factors (from Figure 8-2)		Initial Storm (half street)		Major Storm (full street)	
	Initial Storm	Major Storm	Theoretical Capacity (cfs)	Allowable Capacity (cfs)	Theoretical Capacity (cfs)	Allowable Capacity (cfs)
0.004	0.500	0.500	7.2	3.6	356	178
0.005	0.650	0.650	8.0	5.2	398	259
0.006	0.800	0.800	8.8	7.0	436	349
0.008	0.800	0.800	10.1	8.1	504	403
0.009	0.800	0.800	10.7	8.6	534	427
0.010	0.800	0.800	11.3	9.0	563	450
0.020	0.800	0.700	16.0	12.8	797	558
0.040	0.610	0.500	22.6	13.8	1126	563
0.060	0.410	0.375	27.7	11.4	1380	518
0.080	0.280	0.270	32.0	9.0	1593	430

TABLE 8-1B

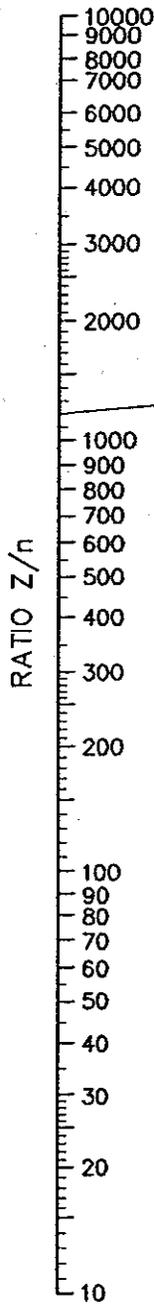


EQUATION: $Q = 0.56 \left(\frac{Z}{n}\right) s^{1/2} d^{8/3}$

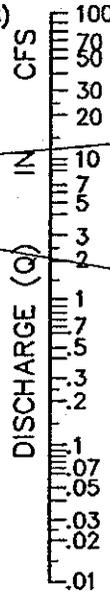
n IS ROUGHNESS COEFFICIENT IN MANNING FORMULA APPROPRIATE TO MATERIAL IN BOTTOM OF CHANNEL Z IS RECIPROCAL OF CROSS SLOPE

REFERENCE: H.R.B. PROCEEDINGS 1948, PAGE 150, EQUATION (14)

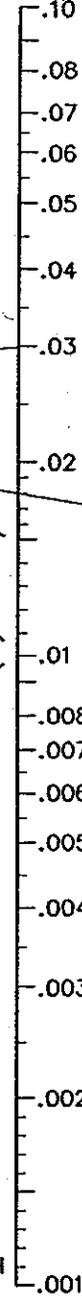
EXAMPLE: (SEE DASHED LINES)
 GIVEN $S = 0.03$
 $Z = 24$
 $n = .02$
 $d = 0.22$
 FIND: $Q = 2.0$ CFS



TURNING LINE



SLOPE OF CHANNEL (S) IN FT/FT

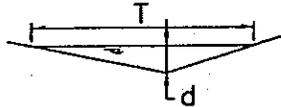


DEPTH AT CURB OR DEEPEST POINT

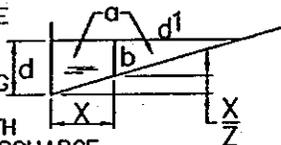


INSTRUCTIONS
 1. CONNECT Z/n RATIO WITH SLOPE (S) AND CONNECT DISCHARGE (Q) WITH DEPTH (d). THESE TWO LINES MUST INTERSECT AT TURNING LINE FOR COMPLETE SOLUTION.

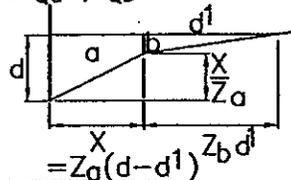
2. FOR SHALLOW V-SHAPED CHANNEL AS SHOWN USE NOMOGRAPH WITH $Z = \frac{T}{P}$



3. TO DETERMINE DISCHARGE Q_x IN PORTION OF CHANNEL HAVING WIDTH X : DETERMINE DEPTH d FOR TOTAL DISCHARGE IN ENTIRE SECTION a . THEN USE NOMOGRAPH TO DETERMINE Q_b IN SECTION b FOR DEPTH $d' = d - (\frac{X}{Z})$



4. TO DETERMINE DISCHARGE IN COMPOSITE SECTION: FOLLOW INSTRUCTION 3. TO OBTAIN DISCHARGE IN SECTION a AT ASSUMED DEPTH d ; OBTAIN Q_b FOR SLOPE RATIO Z_b AND DEPTH d' . THEN $Q_t = Q_a + Q_b$

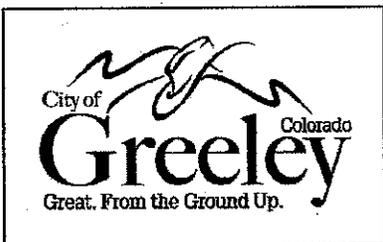


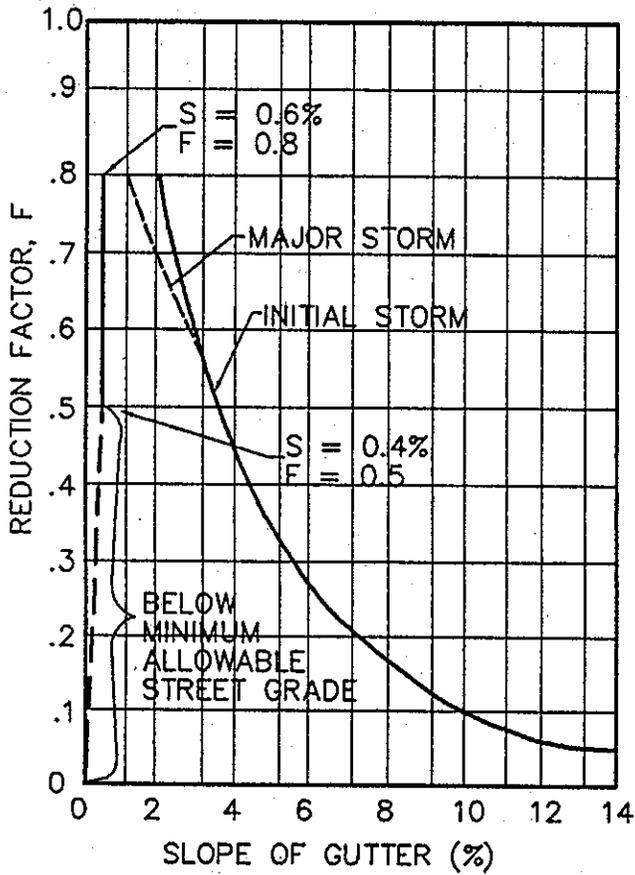
NOMOGRAPH FOR FLOW IN TRIANGULAR GUTTERS

FIGURE 8-1

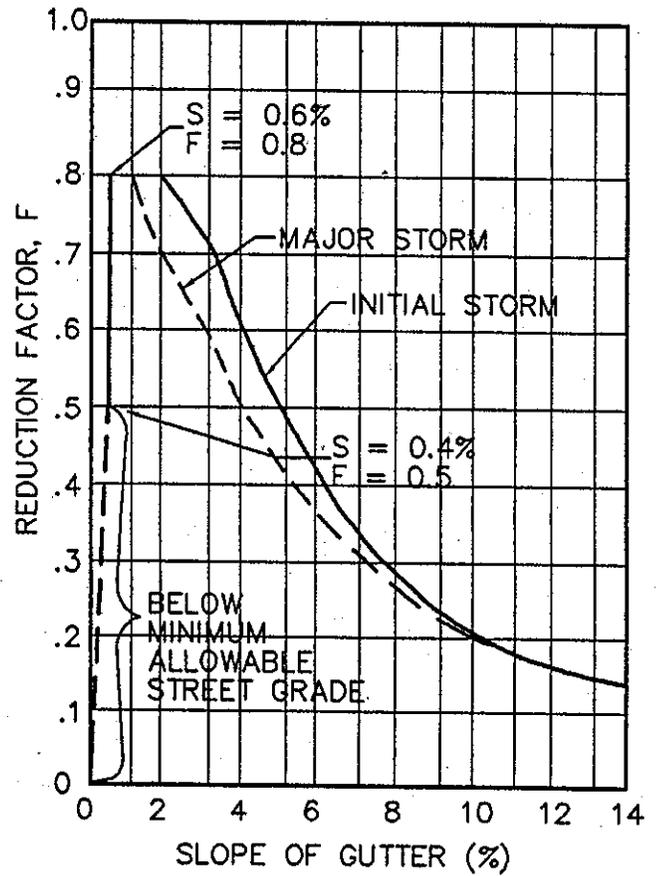
PUBLIC WORKS DEPARTMENT
 STORMWATER MANAGEMENT DIVISION
 1001 NINTH AVENUE GREELEY, COLORADO 80631

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 REVISED NOV 1997





REDUCTION FACTOR FOR ALLOWABLE GUTTER CAPACITY WHEN APPROACHING AN ARTERIAL STREET

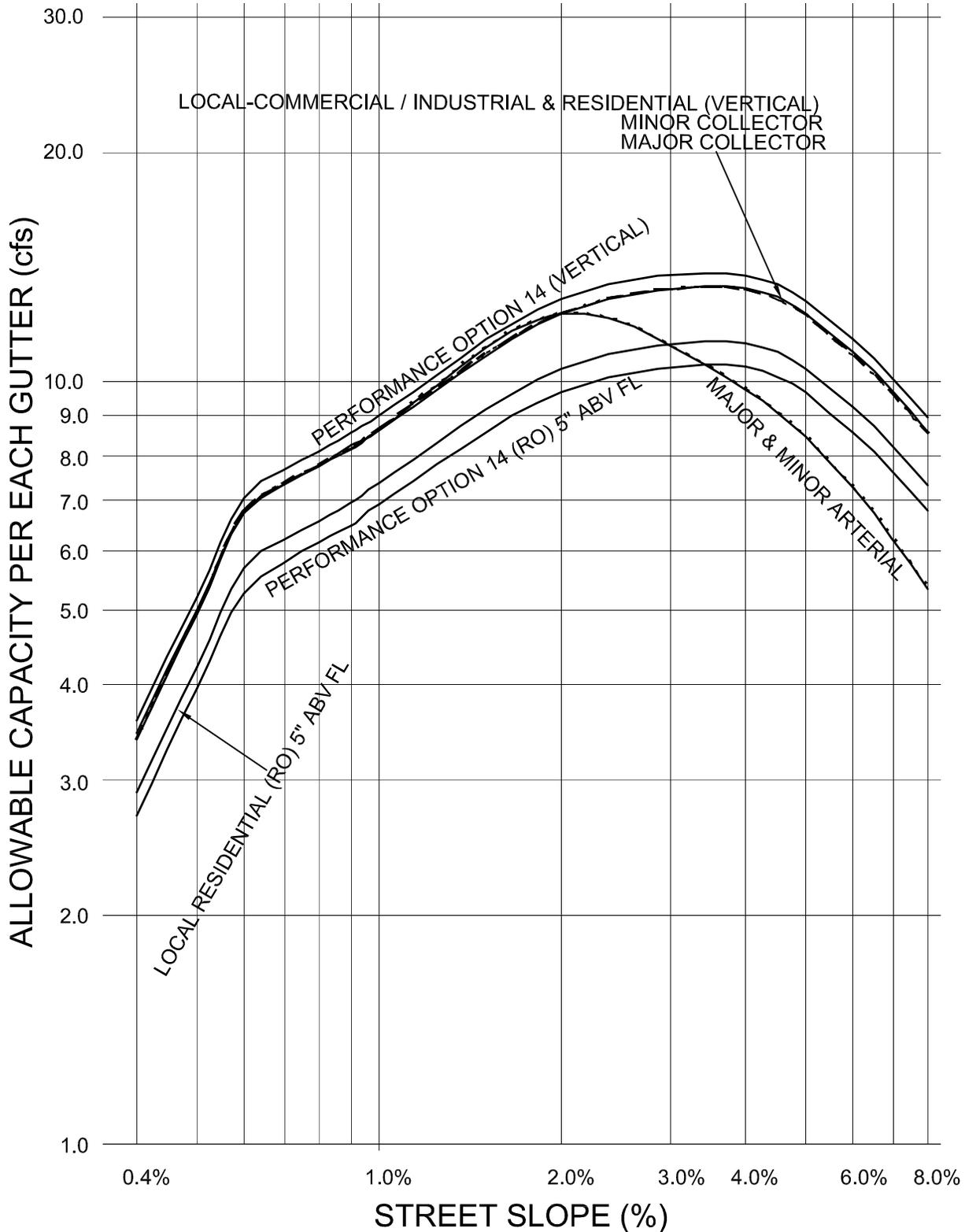


REDUCTION FACTOR FOR ALLOWABLE GUTTER CAPACITY LOCAL AND COLLECTOR STREETS

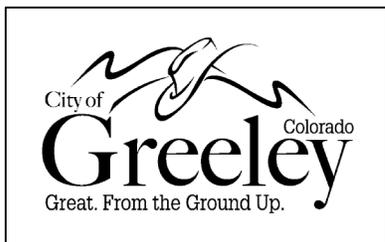
GUTTER CAPACITY REDUCTION CURVES

FIGURE 8-2

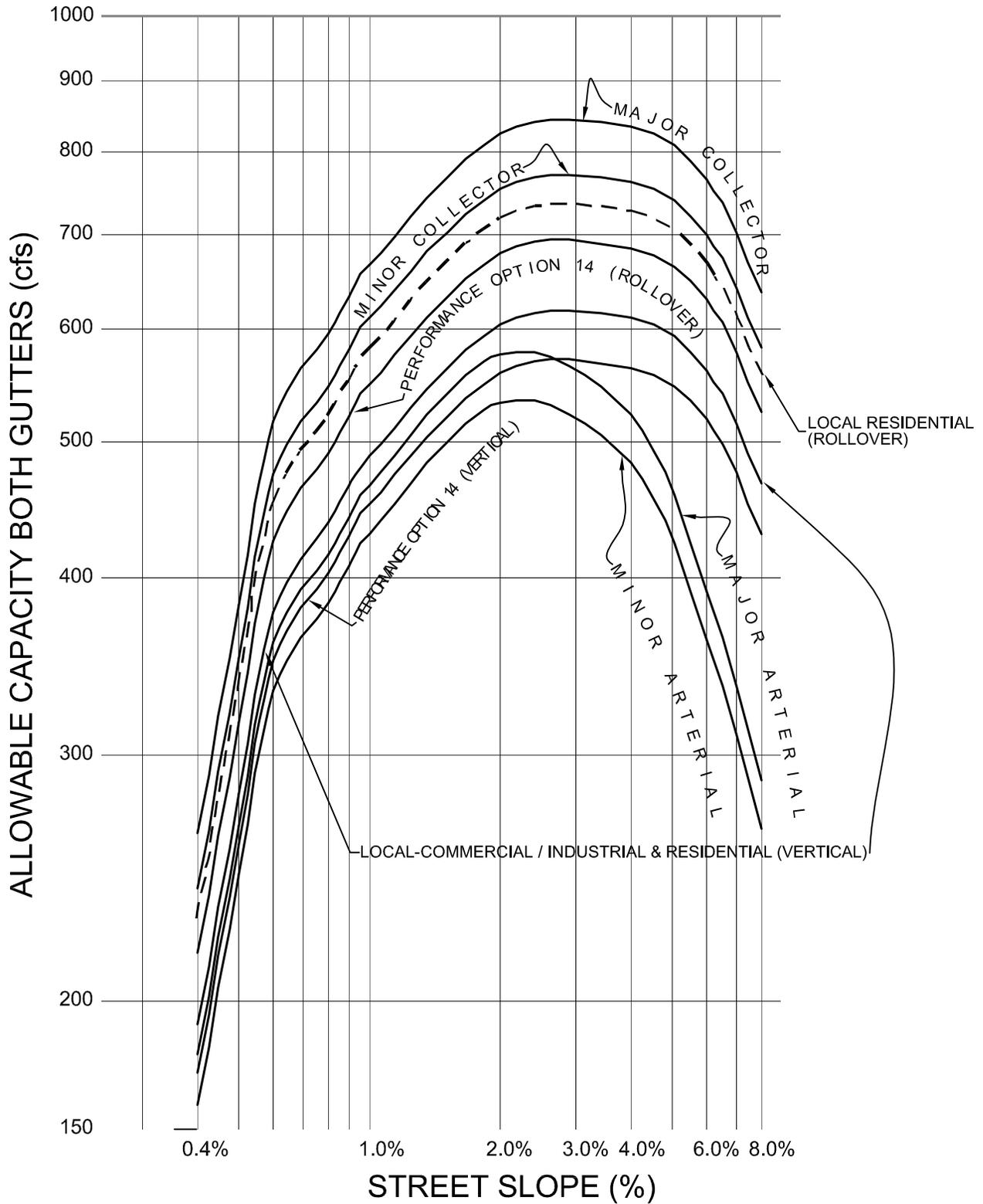
INITIAL STORM



1. THE GRAPH WAS CREATED USING FLOW MASTER WITH MANNING'S FORMULA. THE IMPROVED LOTTER'S METHOD WAS USED FOR THE WEIGHTED ROUGHNESS METHOD. STREET CROSS SECTIONS FOR EACH SLOPE AND EACH TYPE OF STREET LISTED ARE SHOWN.
 ROUGHNESS FACTOR FOR PAVEMENT AND CONCRETE $n = .016$
 ROUGHNESS FACTOR FOR LANDSCAPE AND GRASS AREAS $n = .033$
 2. FIGURE INCLUDES REDUCTION FACTOR FOR ALLOWABLE STREET CAPACITY.



MAJOR STORM



1. THE GRAPH WAS CREATED USING FLOW MASTER WITH MANNING'S FORMULA. THE IMPROVED LOTTER'S METHOD WAS USED FOR THE WEIGHTED ROUGHNESS METHOD. STREET CROSS SECTIONS FOR EACH SLOPE AND EACH TYPE OF STREET LISTED ARE SHOWN.

ROUGHNESS FACTOR FOR PAVEMENT AND CONCRETE $n = .016$

ROUGHNESS FACTOR FOR LANDSCAPE AND GRASS AREAS $n = .033$

2. FIGURE INCLUDES REDUCTION FACTOR FOR ALLOWABLE STREET CAPACITY.

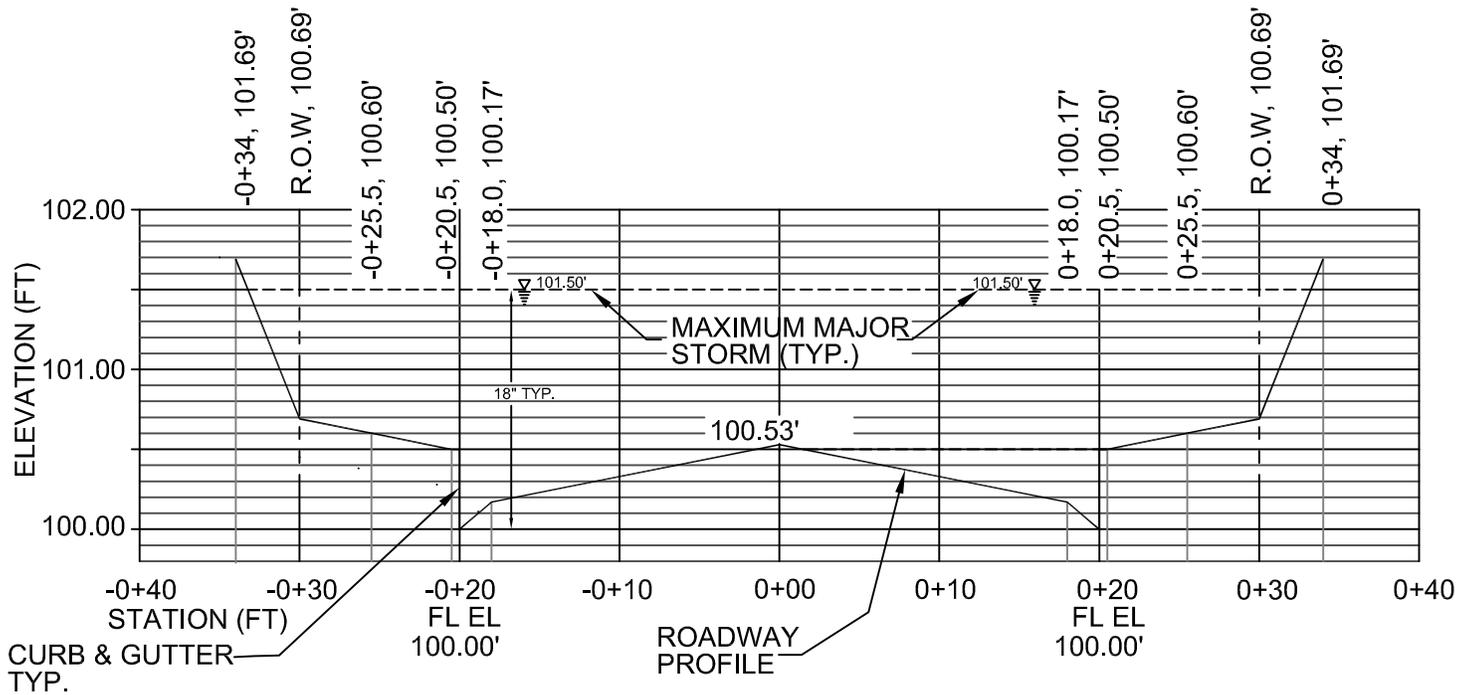
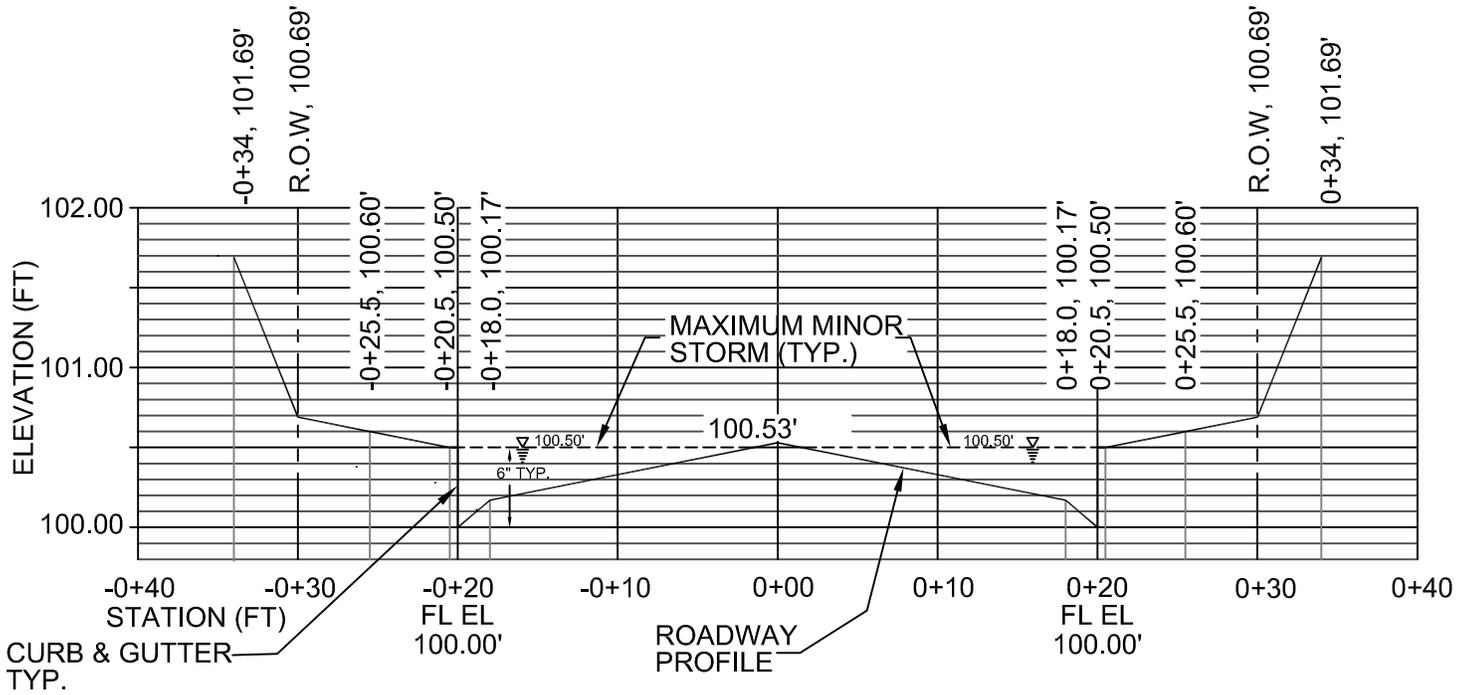
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STORMWATER MANAGEMENT DIVISION

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FIGURE 8-4

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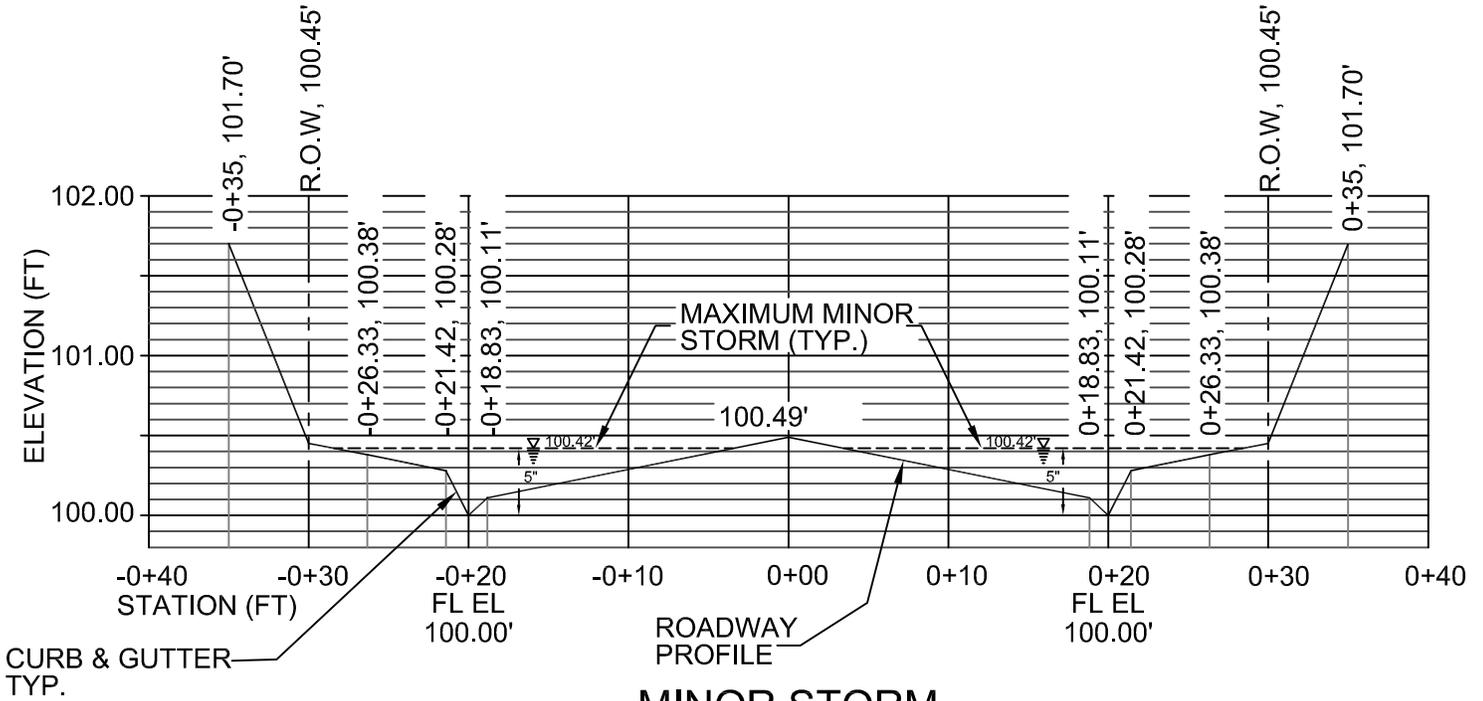




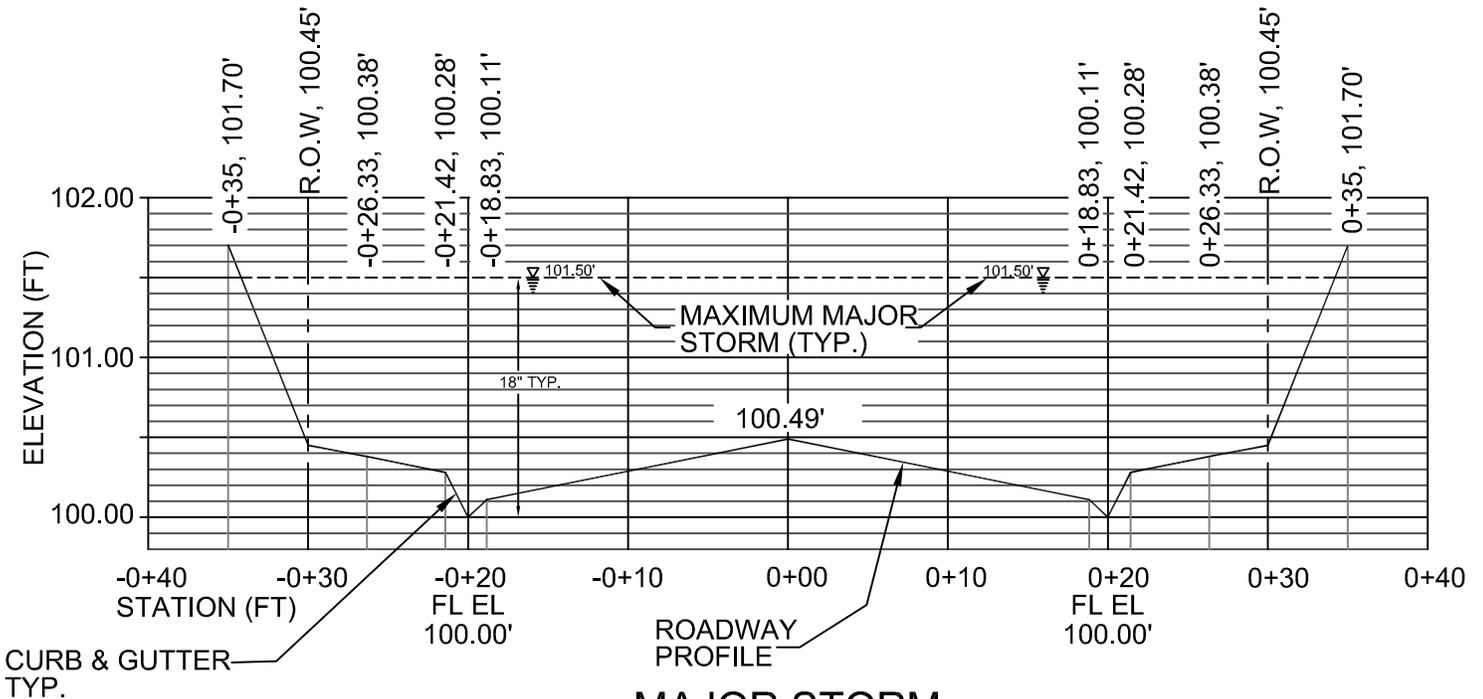
STANDARD ROADWAY SECTION

FIGURE 8-5





MINOR STORM
LOCAL COMMERCIAL/INDUSTRIAL & RESIDENTIAL
ROLLOVER CURB WITH 5" OF WATER DEPTH AT FL

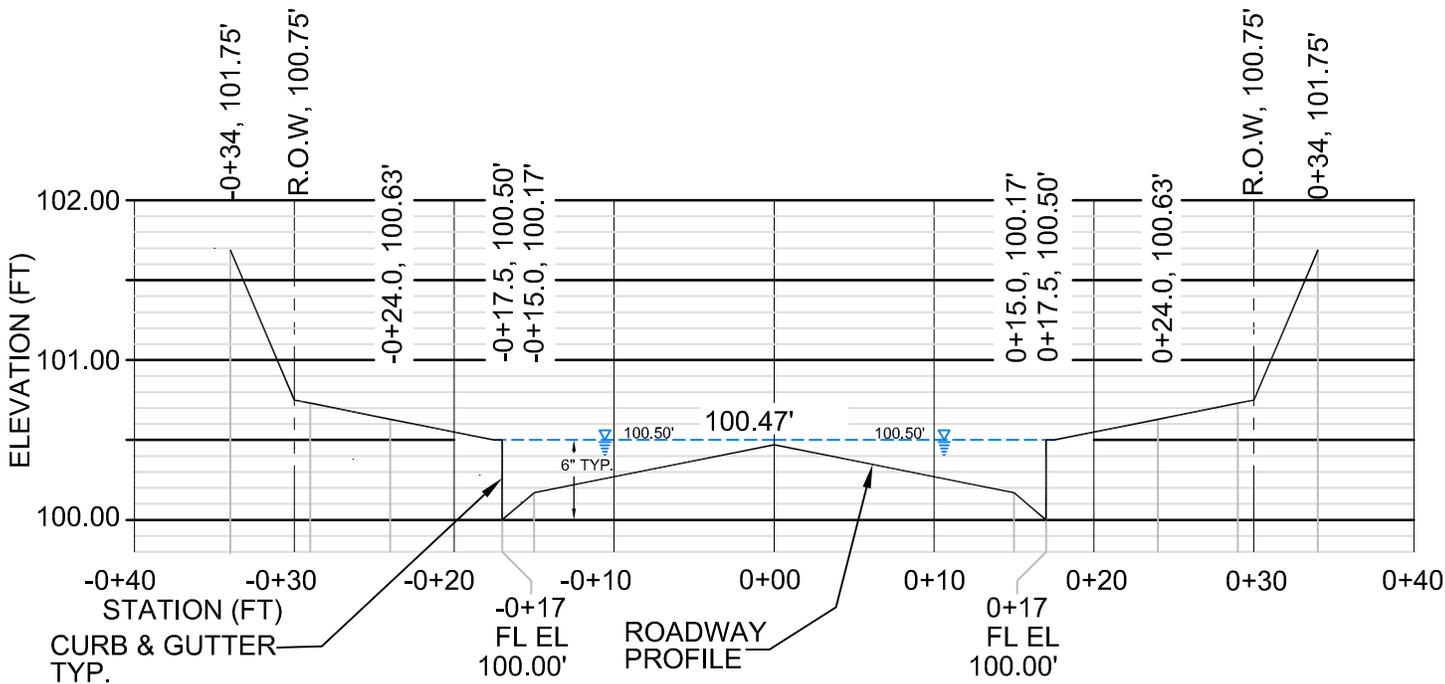


MAJOR STORM
LOCAL COMMERCIAL/INDUSTRIAL & RESIDENTIAL
ROLLOVER CURB WITH 18" OF WATER DEPTH AT FL

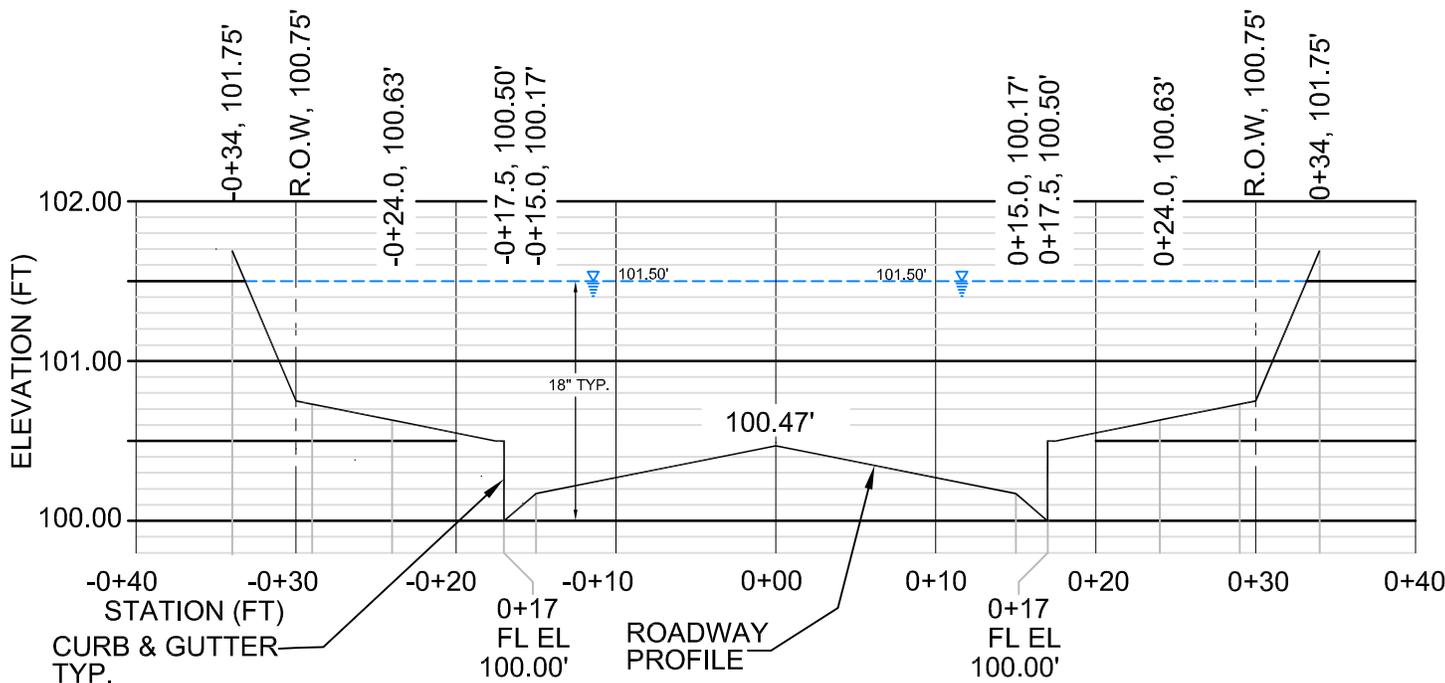
STANDARD ROADWAY SECTION

FIGURE 8-6





**MINOR STORM
PERFORMANCE OPTION 14
VERTICAL CURB WITH 6" OF WATER ABOVE FL**

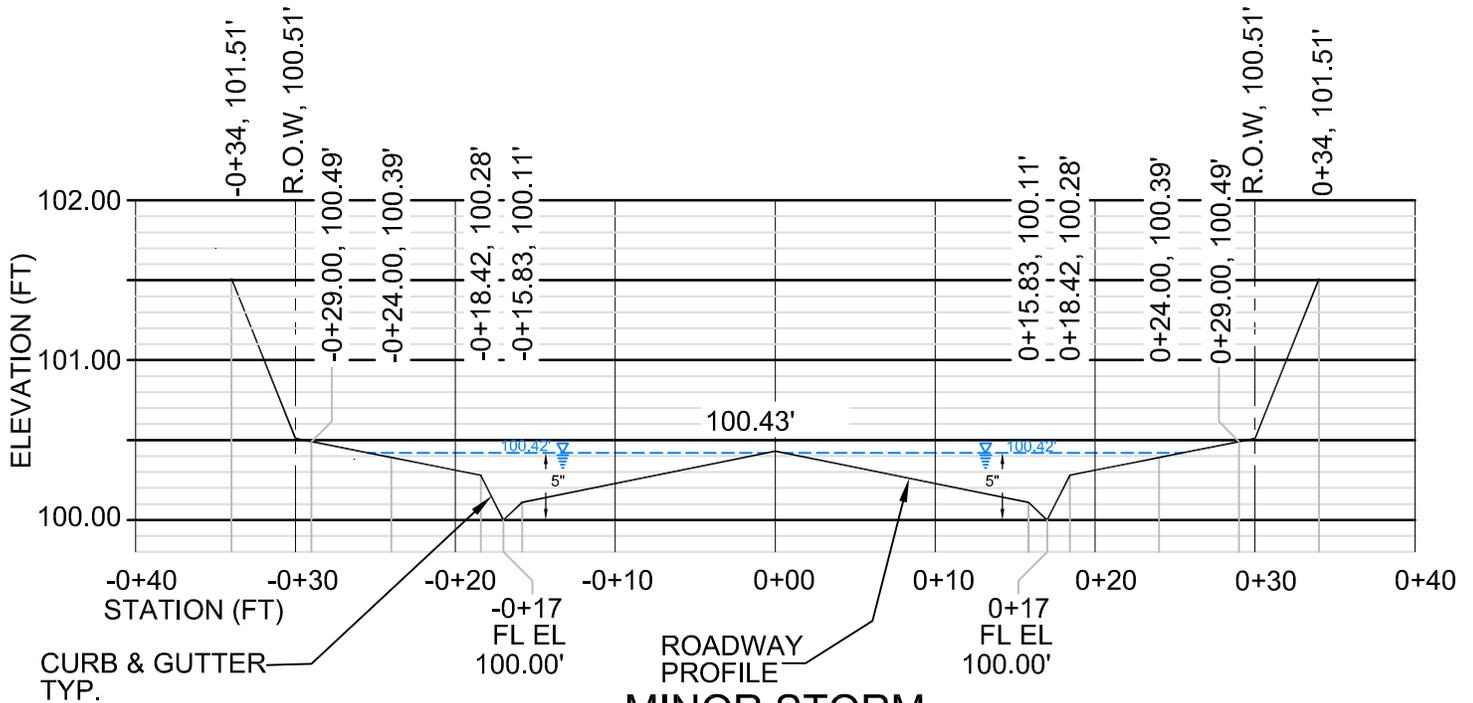


**MAJOR STORM
PERFORMANCE OPTION 14
VERTICAL CURB WITH 18" OF WATER ABOVE FL**

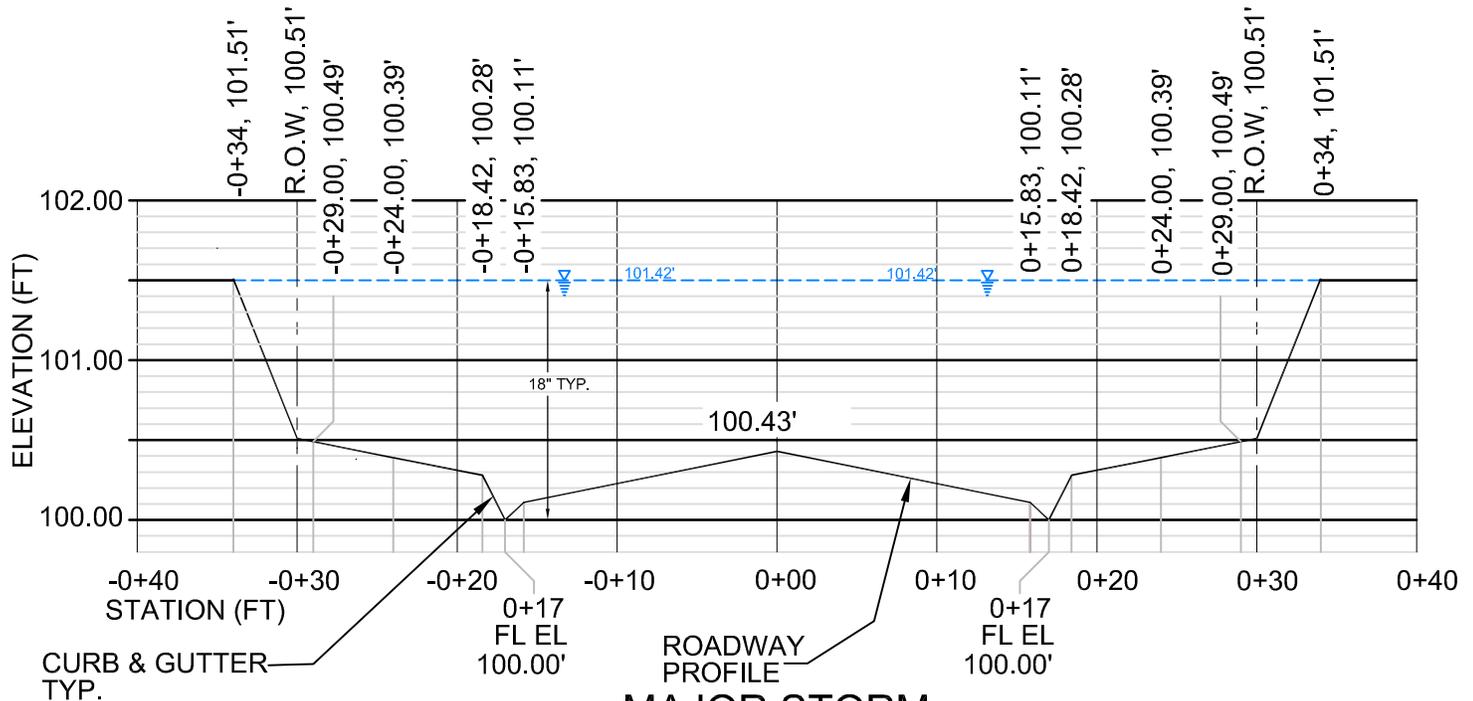
STANDARD ROADWAY SECTION

FIGURE 8-7





**MINOR STORM
PERFORMANCE OPTION 14
ROLLOVER CURB WITH 5" OF WATER DEPTH AT FL**

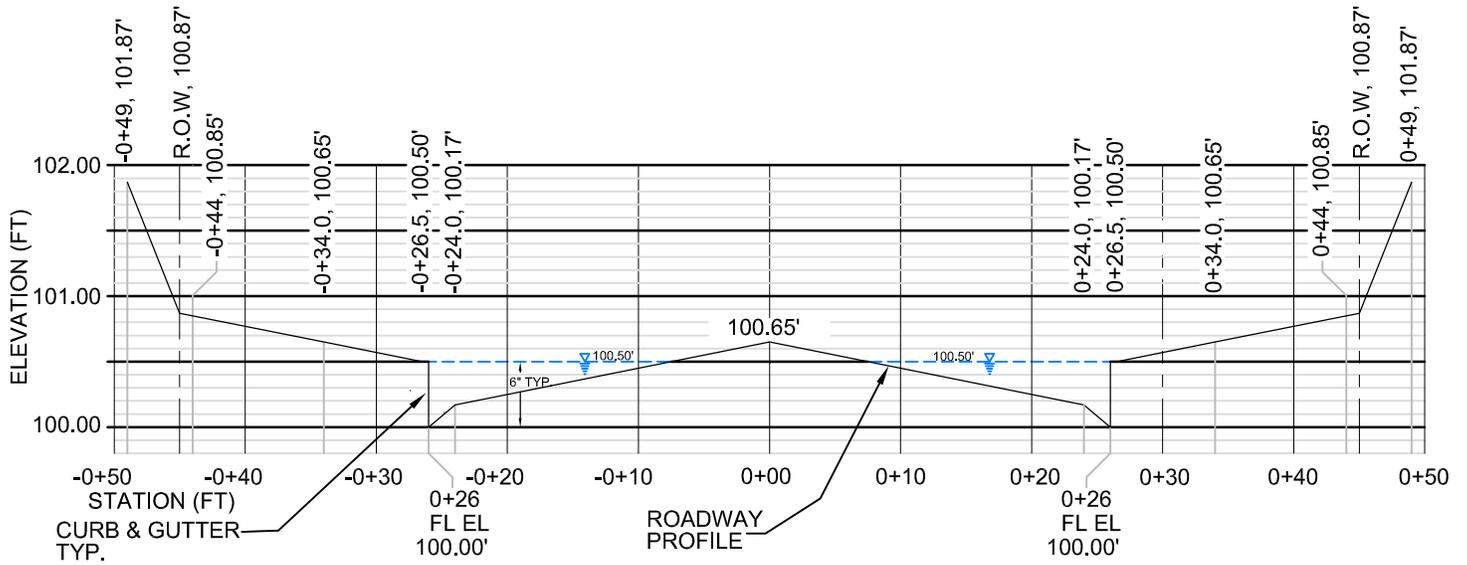


**MAJOR STORM
PERFORMANCE OPTION 14
ROLLOVER CURB WITH 18" OF WATER DEPTH AT FL**

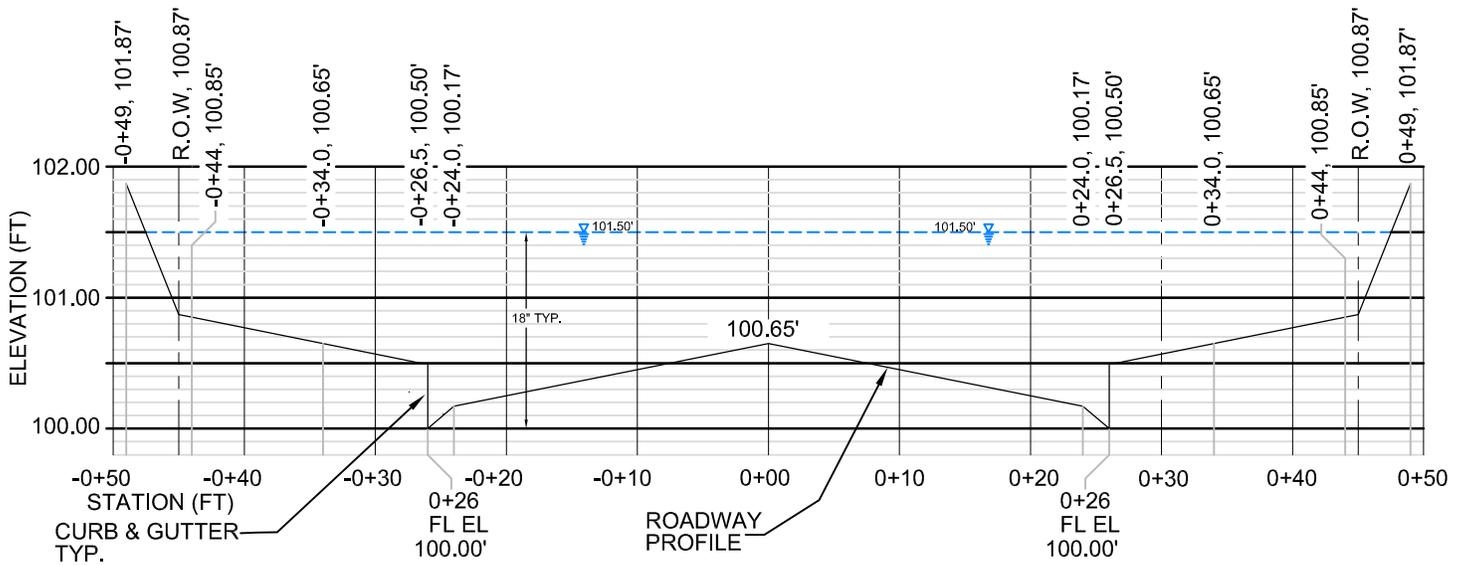
STANDARD ROADWAY SECTION

FIGURE 8-8





MINOR STORM
MINOR COLLECTOR
VERTICAL CURB WITH 6" OF WATER ABOVE FL

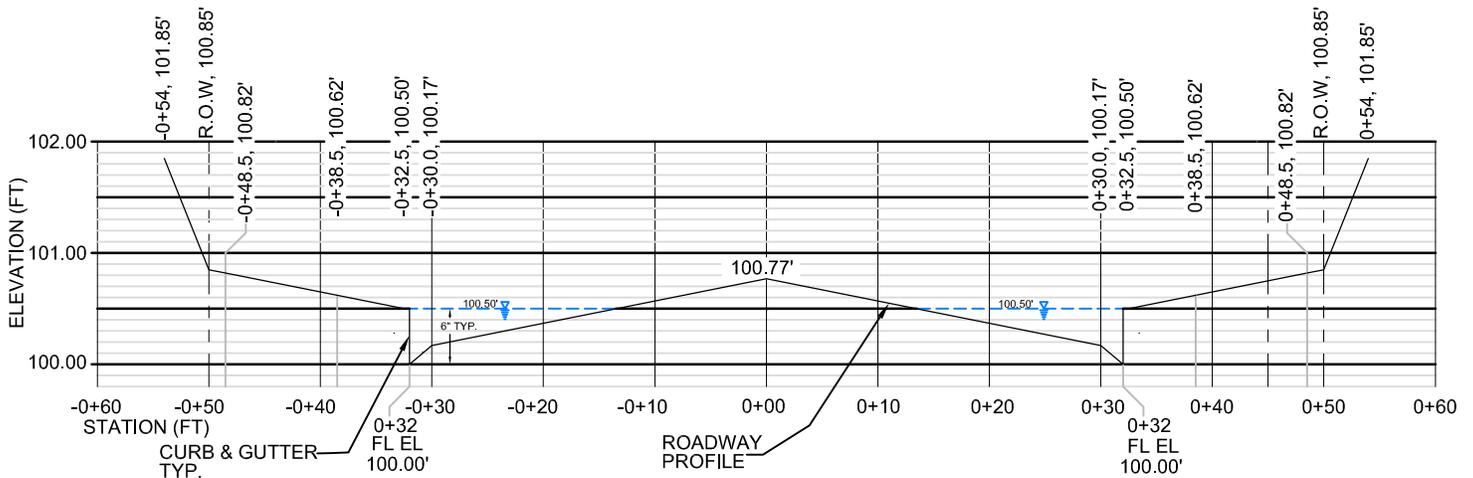


MAJOR STORM
MINOR COLLECTOR
VERTICAL CURB WITH 18" OF WATER ABOVE FL

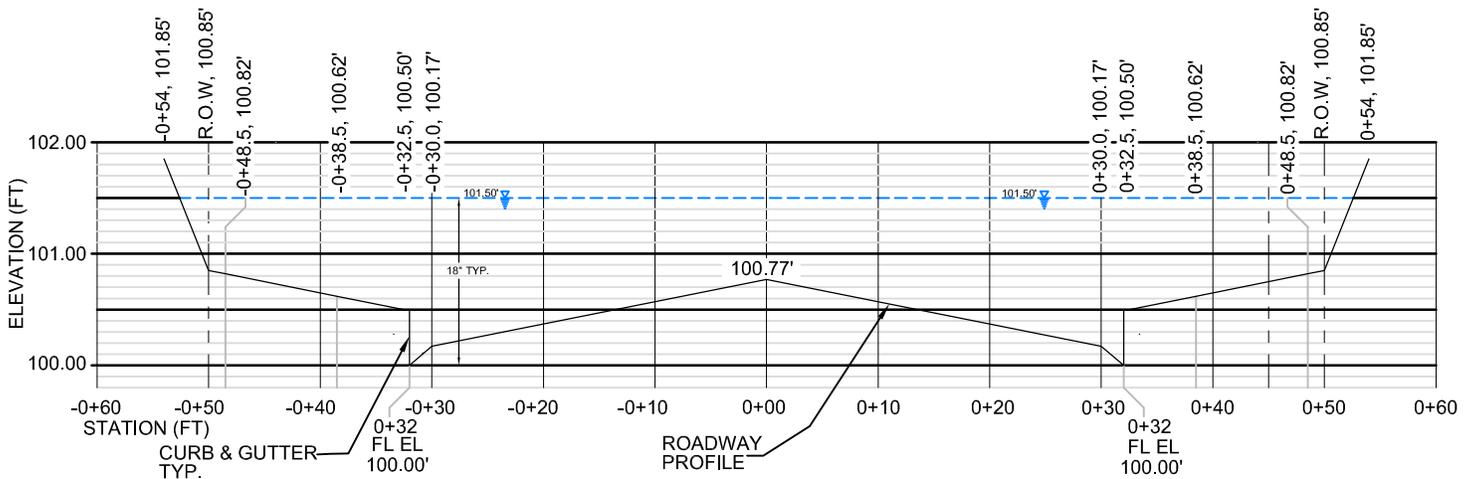
STANDARD ROADWAY SECTION

FIGURE 8-9





MINOR STORM
MAJOR COLLECTOR
VERTICAL CURB WITH 6" OF WATER ABOVE FL



MAJOR STORM
MAJOR COLLECTOR
VERTICAL CURB WITH 18" OF WATER ABOVE FL

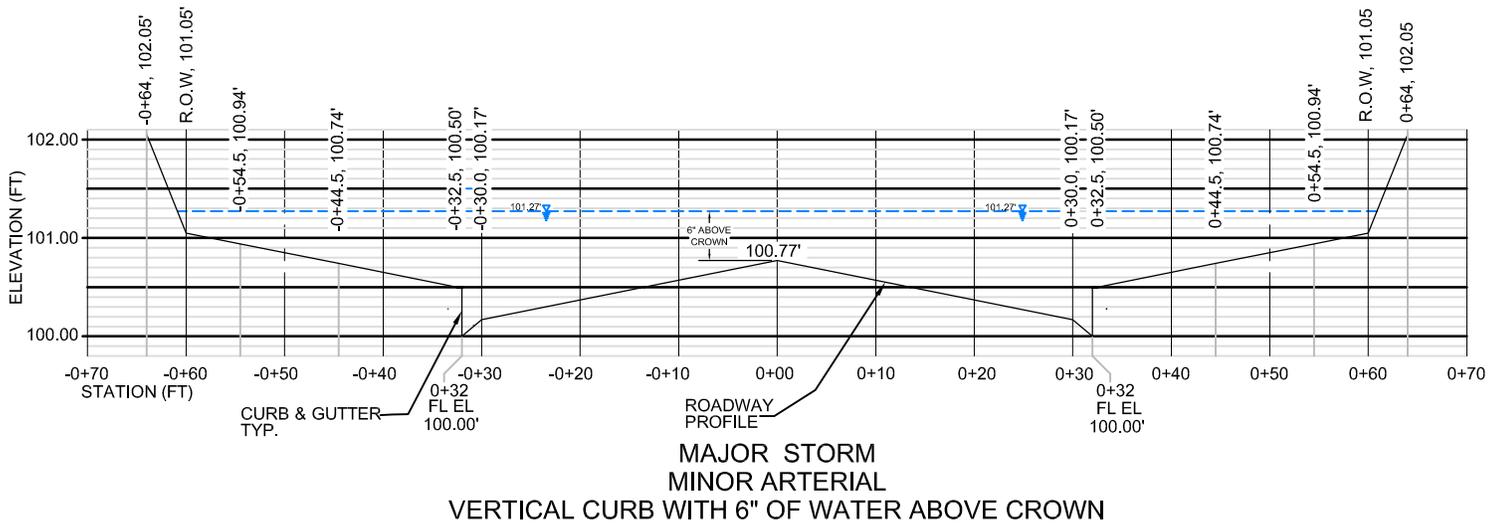
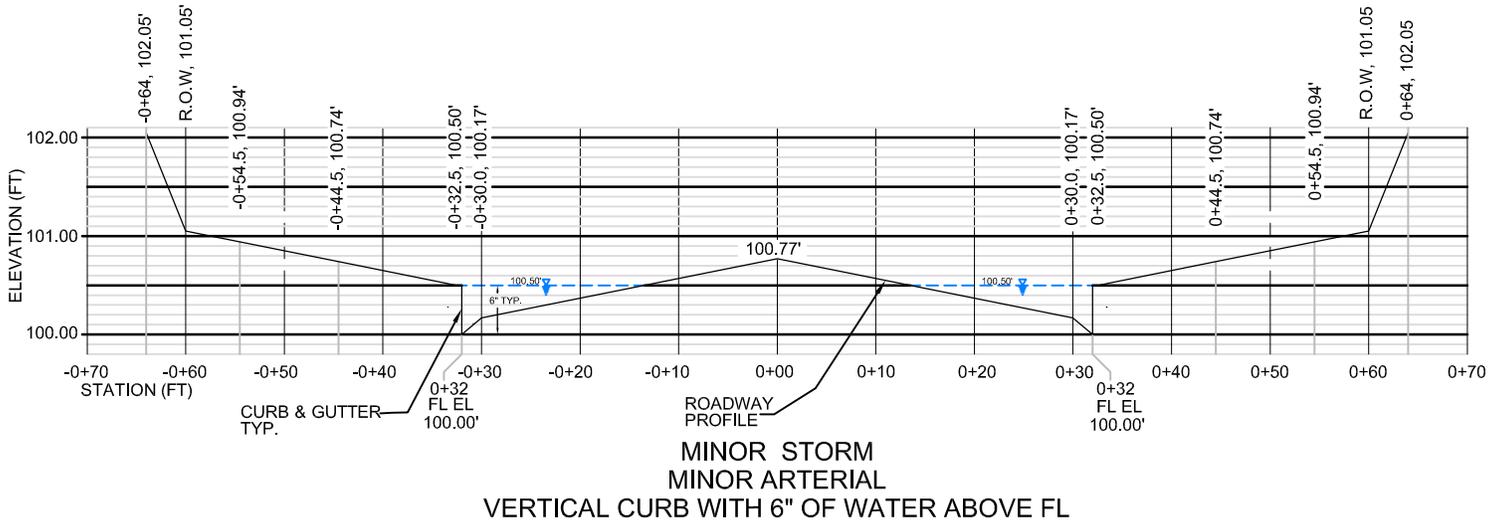


STANDARD ROADWAY SECTION

FIGURE 8-10

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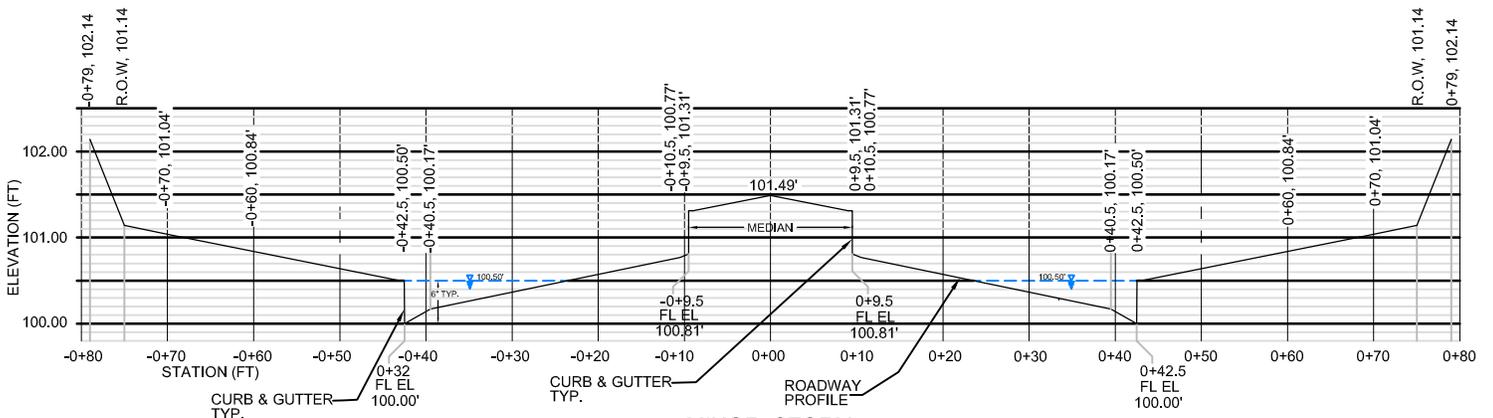
STANDARD ROADWAY SECTION

FIGURE 8-11

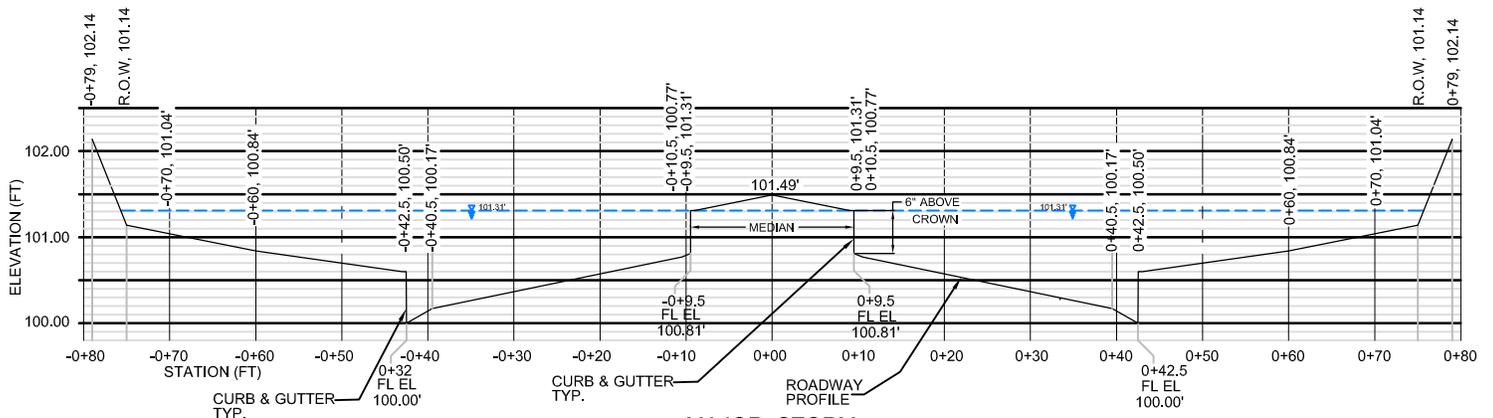
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MINOR STORM
MAJOR ARTERIAL
VERTICAL CURB WITH 6" OF WATER ABOVE FL



MAJOR STORM
MAJOR ARTERIAL
VERTICAL CURB WITH 6" OF WATER ABOVE CROWN

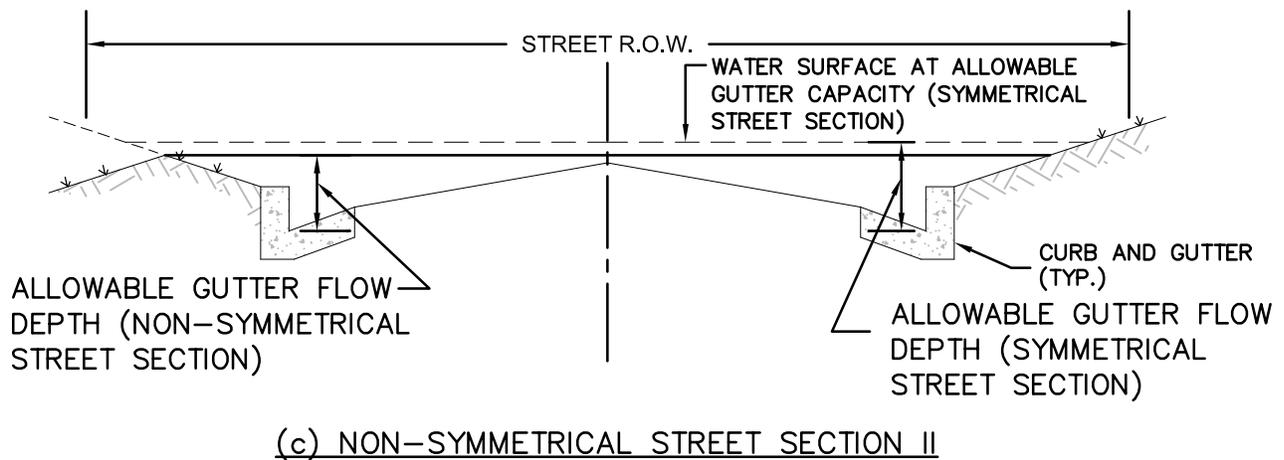
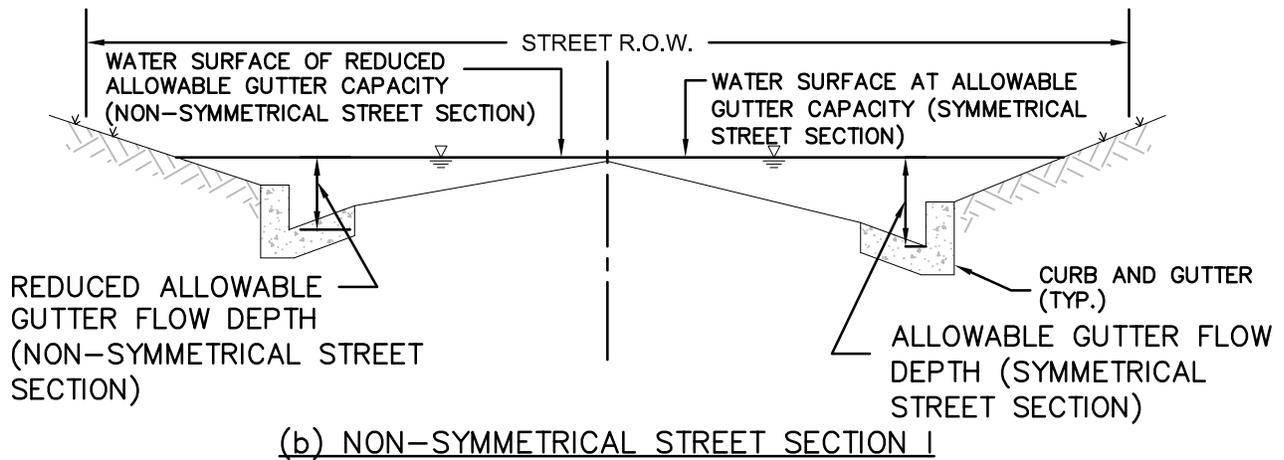
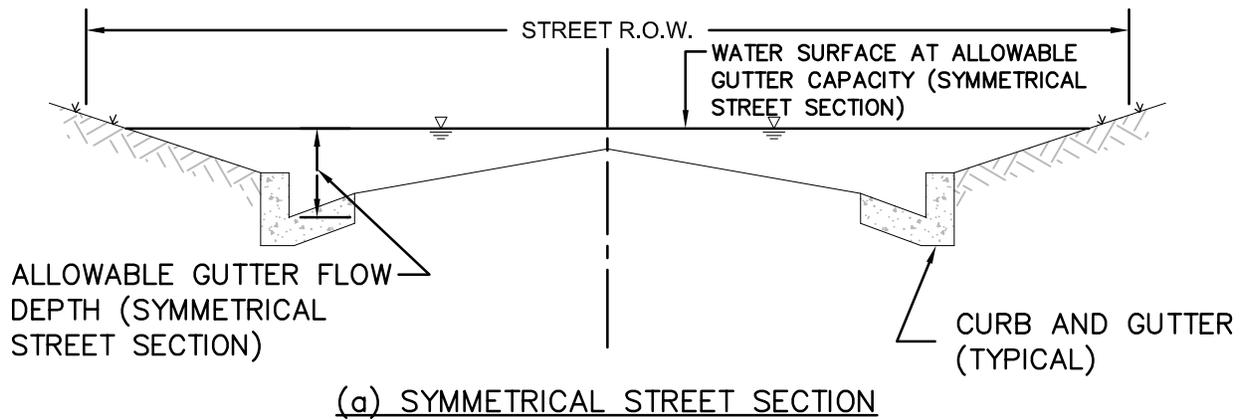
STANDARD ROADWAY SECTION

FIGURE 8-12



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STORMWATER MANAGEMENT DIVISION
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NOTE: FOR NON-SYMMETRICAL STREET SECTION, ADJUST THE TOTAL GUTTER CAPACITY BY REDUCING THE ALLOWABLE GUTTER CAPACITY FOR THE GUTTER WITH THE HIGHER FLOWLINE OR FOR THE ENTIRE SECTION WHEN PROPERTY LINE SLOPES ARE DIFFERENT.

ADJUSTMENT FOR GUTTER CAPACITY WITH NON-SYMMETRICAL STREET SECTION MAJOR STORM

FIGURE 8-13

